IMPACT RESISTANCE OF RUBBERIZED CONCRETE

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ABSTRACT

In the present paper, an experimental work was carried out to study the effect of ground waste tire rubber addition on the mechanical properties and impact resistance of normal strength concrete (NSC) with and without silica fume. Three different volume ratios of crumb rubber (10%, 20%, and 30%) were used as a partial replacement of sand. Two different sizes of cylinders were tested under indirect tension test to study the size effect. Impact compression according to ACI Committee 544's repeated drop-weight impact test was carried out on discs of 150 mm diameter and 63 mm height. Test results showed that, the replacement of fine aggregate by 10% crumb rubber caused a small decrease in concrete compressive strength. The presence of crumb rubber of small size in concrete increased its resistance to crack initiation under impact load. The rubber of small size has no particle bridging effect, hence, the mode of failure of rubberized concrete under static and impact compression was the same as that of plain concrete. The ACI impact test failed to differentiate between the impact resistance of rubberized concretes with different rubber contents. يتعرض البحث بالدراسة المعملية لتأثير إضافة مخلفات مطاط الإطارات المطحون علسي الخسواص الميكانيكيسة ومقاومة الصدم للخرسانة العادية المقاومة في وجود أوعدم وجود عبّار السيلكا. وقد تم استخدام المطاط المطحون كإحلال جزئي للرمل بنسب حجميه تساوى ١٠٪, ٢٠٪, ٣٠٪ بالإضافة إلى الخلطة بدون مطاط. وتمت دراسة تأثير حجم العينة على مقاومة الثند للخرسانة. وتم إجراء اختبار الصدم طبقا لمعهد الأبحاث الأمريكي على عينات اسطوانية قطرها ١٥٠ مم وارتفاعها ٦٣ مم. وقد أظهرت النتائج أن إحلال ١٠% من حجـــم الرمــــل بالمطـــاط المطحون يقلل مقاومة الضغط بنسبة صغيرة كما أن إضافة المطاط المطحون ذات الحجم الصغير للخرسانة يحسن من مقاومتها لبدء الشروخ تحت تأثير أحمال الصدم. كما أوضحت النتائج أن نمط الانهيار تحت تاثير الأحمسال الاستاتيكية وأحمال الصدم للخرسائة المصنوعة من المطاط ذات الأحجام الصغيرة لايختلف عسن نظيره فسي المربكة المعاددة المربكة المربكسي علسى المربكة المربكة المحاددة وقد أوضحت النتائج أيضا عدم مقدرة طريقة اختبار الصدم لمعهد الابحساث الأمريكسي علسى التغريق بين مقاومة الصدم للخرسانة المصنوعة بنسب مختلفة من المطاط.

Keywords: Rubberized concrete; Impact resistance; Drop weight impact test; Silica fume

1. INTRODUCTION

Waste tires pose a health hazard since tire piles are excellent breeding grounds for mosquitoes. Because of the shape and impermeability of tires, they may hold water for long periods providing sites for mosquito larvae development. Waste tires also pose a serious fire hazard since waste tires and waste tire stockpiles are difficult to ignite. However, once ignited tires burn very hot and are very difficult to extinguish. This is due to the 75% void space present in a whole waste tire, which makes it difficult to quench the tires with water or to eliminate the oxygen supply. In addition, the doughnut-shaped tire casings allow air drafts to stoke the fire. A large tire fire can smolder for several weeks or even months, sometimes with dramatic effect on the surrounding environment. An end-of-life tire is a used tire that cannot or is not reused for its originally intended purpose and is not retreated. Such tires may have a further use as a raw material for other processes or be destined for final disposal. End-of-life tires are called "scrap tires" in the United States [1].

The possibility of making concrete tough has been generally pursued by introducing rubber phases among the traditional components (cement, water and aggregates) and this idea has been largely investigated using, for this purpose, recycled grinded tire rubbers [2-7]. Different kinds of tires have been employed as partial substitute of natural aggregates in concrete: scrap tires obtained by simple grinding without further purifications thus including steel and textile fibers in their composition [2,4], crumb rubber obtained by cryogenic process [2], milled tire rubbers treated with sodium hydroxide solution to achieve a better adhesion with the cement paste [7], scrap truck tire rubber [5], tires tread [3], etc. However, regardless the different nature, size and composition of used tire rubbers, a meaningful decrease in concrete compressive strength with the increasing amount of rubber phase in the mixture was always detected. Although the so far obtained rubberized concrete generally shows tougher behavior with a gradual failure of the samples than traditional concrete; it generally does not exhibit suitable

compressive strength for structural applications [8]. The purpose of aggregating rubber is to increase concrete's flexibility, elasticity, and capacity to absorb energy. According to the results of the experiments, it was determined that the addition of rubber aggregate into concrete does in fact increase concrete's impact resistance. This resistance is derived from rubberized concrete's increased ability to absorb energy, safety, and insulate sound during impact. Furthermore, the increase in these properties became more prominent in the concrete samples aggregated with larger rubber aggregate. These altered characteristics are attributed to aggregated rubber because of its fiber structure, which gives the concrete its flexibility and capacity to take in strokes. This increase in elasticity and ability to absorb energy greatly reduces the damage incurred by vehicles colliding with parapets in highway barriers. With this in mind, vehicle impacts taking place with these concrete barriers have been approached as collision problems [9].

Despite the significant increase in the impact resistance of concrete due to the addition of fiber or rubber, researchers and designers are not able to use impact resistance as a design parameter, simply because it cannot be fully quantified due to the lack of a standard impact test for concrete. The absence of a standard impact test prompted researchers to propose their own impact tests to estimate the impact resistance of concrete, either quantitatively or qualitatively. Some of these tests are relatively difficult to perform and require sophisticated equipment. None of these tests, however, have been claimed to be a standard test due to the lack of statistical data on the variation of the results [10]. In this regard, ACI Committee 544 [11] has proposed a drop weight impact test to demonstrate the relative brittleness and to quantify the impact resistance of fiber-reinforced concrete (FRC). The test is widely used because it is simple and economical. The results obtained from this test are often noticeably scattered. The large variation in impact resistance as determined from this test is reported in the literature for different types of FRC. Large variation is a common problem in impact testing, and it is difficult to devise systems that give reproducible results. This might be attributed to the nature of the impact process itself and the number of factors controlling the impact resistance compared with other mechanical properties [10, 12]. The main objective of the present work is to study the effect of ground waste tire rubber as a partial replacement of sand on the mechanical properties and impact resistance of normal strength concrete (NSC) with and without silica fume. Three different volume ratios of crumb rubber (10%, 20%, and 30%) were used as a partial replacement of the sand.

2. EXPERIMENTAL WORK

All test specimens were fabricated using locally available materials. Type I ordinary Portland cement was utilized. Silica firme from the Ferro silicon alloys plant in Edfo zone, Egypt, was used. All the bags of silica fume had not any indication for the date of production and the expired date. Therefore, the used silica fume was stored in good storing conditions for two years to examine its validity. The sand used was local natural siliceous sand with specific gravity of 2.55. The coarse aggregate used was dolomite with maximum aggregate size of 14 mm and specific gravity of 2.6. A superplasticizer called Adecrite PVF (naphthalene sulphonated compound) was chosen. The used crumb rubber in this research was produced by (El-Nasr Tire Company) by grinding the waste tires with special technique. Table 1 shows the passing percentage of crumb rubber.

In this study, three different volume ratios of crumb rubber (10%, 20%, and 30%) were used as a replacement of the sand and two different cementatious material contents of 443 Kg/m³ cement and (443 Kg/m³ cement + 10% silica fume from cement weight) were used. The concrete mix proportions by weight for the two mixes was (1 cement: 1.34 sand: 2.246 dolomite: 0.34 water: 0.01, by weight of cement+ SF, super-plasticizer). Cubes 150 x 150 x 150 mm were prepared to be tested under static compression. Two different sizes of cylinders of 100 mm diameter and 200 mm height and 150 mm diameter and 300 mm height were prepared to be tested under indirect tension test to study the size effect. On the other hand, discs of 150 mm diameter and 63 mm height were cast to be tested under impact compression according to ACI Committee 544's repeated drop-weight impact test. Concrete specimens were cast in steel moulds. After 24 hours the specimens were de-molded and water cured for 28 days. All specimens were cast and treated under the same environmental conditions.

Table 1 Passing percentage of crumb rubber.

Sieve Opening, mm	Passing %			
5	90			
2.5	54			
1.25	26			
0.2	5			
0.31	1			
0.16	Ö			

Special impact machine was designed and fabricated at the laboratories of Zagazig University adopting the technique of drop weight impact test. The impact machine was manufactured according to ACI recommendations; Fig. 1 shows the test rig. A brief of the test is that, the samples are coated on the

bottom with a thin layer of petroleum jelly or a heavy grease and placed on the base plate within the positioning lugs with the finished face up (if appropriate). The positioning bracket is then bolted in place, and the hardened steel ball is placed on top of the specimen within the bracket. The drop hammer is placed with its base upon the steel ball and held there with just enough down pressure to keep it from bouncing off the ball during the test. The base plate should be bolted to a rigid base, such as a concrete floor or east concrete block. The hammer is dropped repeatedly, and the number of blows required to cause the first visible crack on the top and to cause ultimate failure are both recorded. Ultimate failure is defined as the opening of cracks in the specimen sufficiently so that the pieces of concrete are touching three of the four positioning lugs on the base plate. Results of these tests exhibit a high variability and may vary considerably with the different types of mixtures, fiber contents, etc.

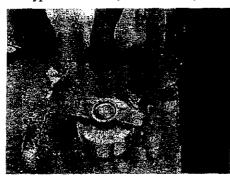






Fig. 1 Impact test

 $A = \delta$

3. RESULTS AND DISCUSSIONS

The mean values of measured compressive strength and splitting tensile strength for different plain and rubberized concretes are presented in Table 2. Generally, the presence of silica fume causes small improvement in the compressive strength. This may be due to the long time of storage. The long storage time of silica fume made it absorb humidity from the surrounding atmosphere and so form large particles (i.e. decreasing fineness). This may affect its physical and chemical effect. There is also a reduction in concrete compressive and tensile strength due to the replacement of sand by crumb rubber with different volume fractions. It is clear that, there is a small reduction in the case of 10% replacement. Therefore, it can be concluded that, the 10% replacement of fine aggregate by crumb rubber has a marginal effect on concrete compressive strength, Although there is a small effect of silica fume addition in PC concrete. the rubberized concrete with silica fume showed higher compressive strength than those without silica fume. This is because the presence of silica fume in the gase of rubberized concrete enhanced the bond strength between the rubber particles and the cement paste [13]. There is no clear trend for the size effect on the indirect tensile strength measured from 10 × 20 cm and 15 \times 30 cm cylinders. The mode of failures for various rubberized concretes under compression indirect tension loads are the same as those of plain concrete. This may be due to small amount of rubber content and the small size of rubber particle in the mixes.

The simplest of the impact tests is the "repeated impact," drop-weight test. This test yields the number of blows necessary to cause prescribed levels of distress in the test specimen. This number serves as a qualitative estimate of the energy absorbed by the specimen at the levels of distress specified. The test can be used to compare the relative merits of different rubber-concrete mixtures and demonstrate the improved performance of rubberized concrete compared to conventional concrete. Figure 2 shows the crack pattern and the associated level of visible damage due to the impact force for either plain concrete or rubberized concrete. It is clear that, all specimens are divided to separate parts under the effect of impact force. No visible cracks were observed in each of the separated parts and no dislocated dolomite particles were found across the fractured surface. This may be due to the good bond between the mortar and the dolomite. Therefore, the favorable crack path is across the dolomite particles not around the surface of the particles. There is no particle bridging found in the case of rubberized concrete due to the small size of the crumb rubber. Particle bridging may be occurred in the case of large crumb rubber.

Table 2 Mechanical properties of plain and rubberized concretes

Crumb Rubber%	0.0%		10%		20%		30%	
Silica Fume%	0.0%	10%	0.0%	10%	0.0%	10%	0.0%	10%
f _{cu} , MPa	37.93	38.44	35,56	38.3	31.85	34.00	30.00	32.30
∫ _{sp[5*30} , MPa	4.53	4.69	3.89	3.04	3.25	3.29	2.67	2.97
f _{sp10*20} , MPa	3.98	3.61	4.08		2.97	4.35	4.14	-



Plain Concrete



Rubberized Concrete

Fig. 2 Mode of failure plain and rubberized concretes under impact load.

Figures 3 and 4 show the number of blows till first crack occurs for nine specimens for the rubberized concrete without and with 10% silica fume addition

respectively. It is clear that, there is a wide discrepancy in the results for all rubber replacement ratios. A wide range in results of about 3-4 times the mean value of the number of blows is observed. This wide range comes from the philosophy of the test itself not from any wrong steps in test procedures. The statistical analysis of the results of first crack is made in Table 3. The addition of silica fume has not clear effect on the resistance of concrete to crack initiation under impact compression load. The replacement of sand by crumb rubber increased the resistance of concrete to crack initiation under impact compression load. The ACI impact test failed to differentiate between the rubberized concretes with different rubber contents.

Figures 5 and 6 show the number of blows till failure occurs for nine specimens for the rubberized concrete without and with 10% silica fume addition respectively. The same behavior was found as in the case of first crack. Table 5 indicates that, the ratio of complete failure to the first crack is slightly above the unity (ranged from 1.03 to 1.14), i.e. the failure takes place rapidly in rubberized concrete. Therefore, we can claim that, the rubber with small size (no particle bridging) has a little effect to delay the crack propagation in concrete. Generally, adding the rubber to the concrete results in a significant increase in the number of blows.

Figures 7 and 8 show the statistical analysis of the results of complete failure for the rubberized concrete without and with 10% silica fume addition respectively. The range and the standard deviation are extremely high.

Table 3 Statistical analysis of number of blows to first crack

	Rubber %	Silica Fume %	Mean (blows)	Range / mean	C.O.V.*
C		0	35	4.83	1.74
SC		10	39	3.69	1.20
R1	10	0	290	3,80	1.21
SR1	10	10	62	4.67	1.80
R2	20	0	67	10.75	3.58
SR2	20	10	177	3.47	1,40
R3	30	0	140	6.90	2.32
SR3	30	10	63	3.95	1.32

^{*} C.O.V is the coefficient of variance

diams

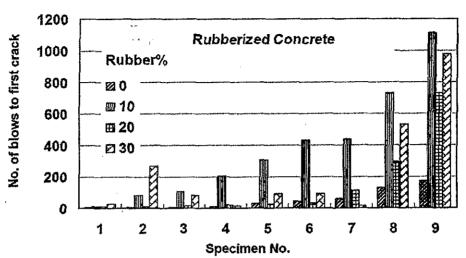


Fig. 3 Number of blows to first crack in rubberized concrete without silica fume

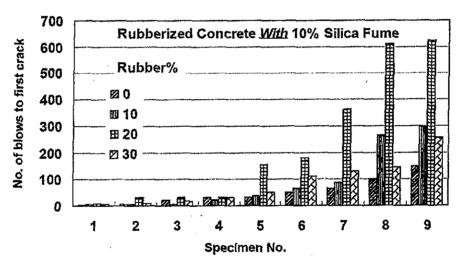


Fig. 4 Number of blows to first crack in rubberized concrete with silica fume.

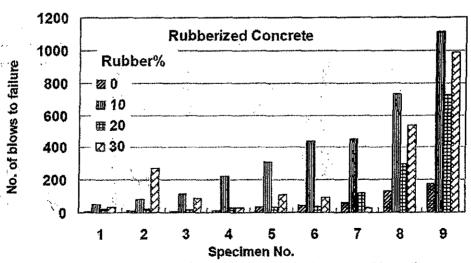


Fig. 5 Number of blows to failure cracks in rubberized concrete without silica

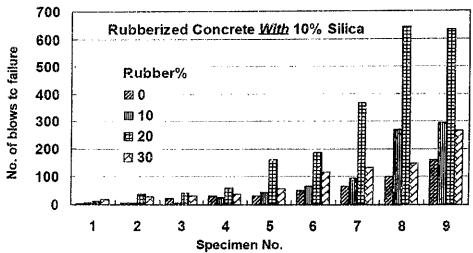


Fig. 6 Number of blows to failure cracks in rubberized concrete with silica fume

Table 5 Statistical analysis of rubberized concrete

		without silica fume				with silica fume		
	-	Mean (blows)	Failure / 1st Crack			Mean (blows)	Failure / 1st Crack	
С	1 st crack	35	1.06	sc	1st crack	39	1	
	Failure	37			Failure	39		
R1	1st crack	290	1.03	SR1	1 st crack	62	1.05	
	Failure	300			Failure	65		
R2	1st crack	67	1.07	1.07 SR2	1 st crack	177	1.07	
	Failure	72				Failure	189	
R3	I st crack	140	1.05	SR3	1 st crack	63	1.14	
	Failure	147			Failure	72	,	

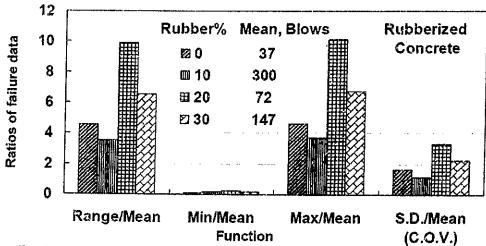


Fig. 7 Statistical analysis of complete failure in rubberized concrete without silica fume

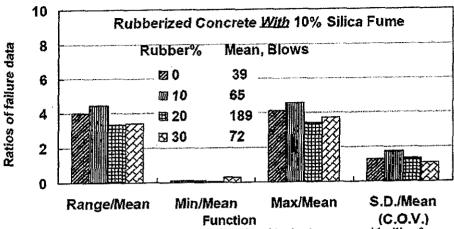


Fig. 8 Statistical analysis of complete failure in rubberized concrete with silica fume

4. CONCLUSIONS

From the experimental results obtained in this study, the following conclusions can be drawn:

- There was a reduction in concrete compressive and tensile strength due to the replacement of sand by crumb rubber with different volume fractions.
- 2. The 10% replacement of fine aggregate by crumb rubber caused a small decrease in concrete compressive strength.
- 3. The presence of crumb rubber of small size in concrete increased the resistance of concrete to crack initiation under impact load.
- 4. The rubber of small size had no particle bridging effect, hence, the mode of failure of rubberized concrete under static and impact compression was the same as that of plain concrete.
- The effect of silica fume addition on the compressive was more obvious in the case of rubberized concrete compared to plain concrete.
- The ACI impact test failed to differentiate between the impact resistance of rubberized concretes with different rubber contents.

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